

Welded-Bolted Light Bracing Connections

Example. Check Slip Resistance.

$$T_u = 150 \text{ kips} \quad \phi = 0.85 \quad \mu = 0.5 \quad D_u = 1.13$$

$$h_f = 1.0 \quad T_b = 28 \text{ kips} \quad n_s = 1 \quad k_{sc} = 0.6974$$

$$\phi R_n = \phi \mu D_u h_f T_b n_s k_{sc} / \text{bolt}$$

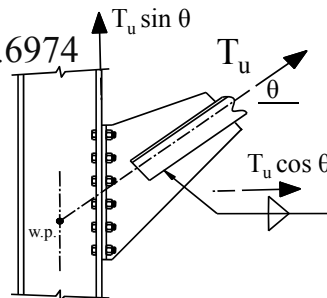
$$= 0.85 \times 0.50 \times 1.13 \times 1.0 \times 28 \times 1 \times 0.6974$$

$$= 9.35 \text{ kips/bolt}$$

$$\sum \phi R_n = 9.35 \times 12 = 112 \text{ k}$$

$$T_u \sin \theta = 150 \sin 40^\circ$$

$$= 96.4 \text{ k} < 112 \text{ k} \quad \text{OK}$$



Heavy Bracing Connections

- Gusset plate is generally welded to beam and bolted to column.
- One new limit state: Gusset Plate Compression Buckling.
- Forces at beam and column determined using the Uniform Force Method.
- **Beam-to-gusset plate weld is increased 25%** to account for non-uniform distribution of forces along weld length.

Heavy Bracing Connection Example

Estimate α and β

α :

Assume 7/16" fillet welds

$$L_w = 616 / (2 \times 1.392 \times 7) = 31.6''$$

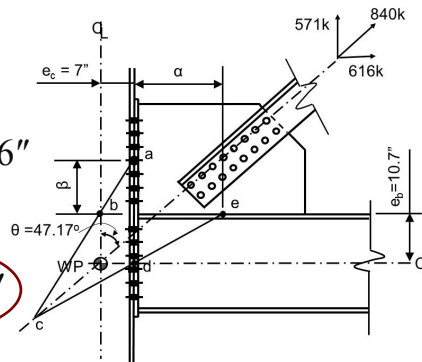
Try $\alpha = 17.5''$

β :

$$\beta = \frac{7.0 + 17.5}{\tan 47.17^\circ} = 10.7 = 12.0''$$

Try 7 rows 7/8" A490

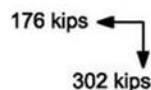
$$\beta = 3 + 3 \times 3 = 12''$$



Heavy Bracing Connection Example

Design Gusset-to-End-Plate Weld

Fillet B.S. E70xx $L_w = 7 \times 3.0 = 21.0''$



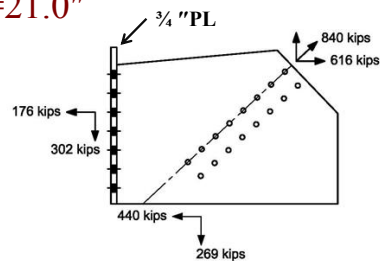
$$R_u = \sqrt{176^2 + 302^2} = 350 \text{ k}$$

$$\theta = \tan^{-1} (176/302) = 30.2^\circ$$

$$D_{req'd} = 350 / [(2 \times 1.392 \times 21.0)(1 + 0.5 \sin^{1.5} 30.2^\circ)]$$

$$= 5.07 - 1/16 \text{ s} \quad \text{Min.} - 5/16$$

Use 3/8" Fillet Welds B.S.



Heavy Bracing Connection Example

Check End-Plate to Col. Flange Bolt Strength

14 – 7/8" A490-X Bolts $A_b = 0.601 \text{ in}^2$ (No Prying)

$$\phi R_{nt} = 0.75 F'_{nt} A_b \text{ per bolt (J3-3a)}$$

$$r_{uv} = 302/14 = 21.6\text{k} < \phi V_n = 37.9\text{k} \text{ OK}$$

$$r_{ut} = 176/14 = 12.6\text{k}$$

$$f_{uv} = 21.6/0.601 = 35.9 \text{ ksi}$$

$$\text{and with } F_{nt} = 113 \text{ ksi } F_{nv} = 84 \text{ ksi}$$

$$F'_{nt} = 1.3F_{nt} - \frac{F_{nt}}{\phi F_{nv}} f_{uv} \leq F_{nt}$$

$$= 1.3 \times 113 - \frac{113}{0.75 \times 84} 35.9 = 82.5 \text{ ksi} < 113 \text{ ksi}$$

$$\phi R_{nt} = 0.75 \times 82.5 \times 0.601 = 37.2\text{k} \geq r_{ut} = 12.6\text{k} \quad \text{OK}$$

