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 Session Description


Session 3: Design of Horizontally Curved Members
July 2, 2018

The session addresses the treatment of horizontally-curved members. Topics include: flexural strength, torsional strength, combined flexural and torsional loads, serviceability, local strength and connections.

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 Learning Objectives

- Identify the limit states for horizontally curved members.
- Identify 3 analysis methods and describe the preferred method.
- Describe the steps of designing a horizontally curved member for flexural and torsional strength.
- Describe the various connection considerations for horizontally curved members.


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Design of Curved Members
Session 3: Design of Horizontally Curved Members
July 2, 2018

Presented by
Bo Dowsell, P.E., Ph.D.
ARC International, LLC
Birmingham, AL




There's always a solution in steel.



There's always a solution in steel.


Design of Horizontally-Curved Members

Session Description




Session Description

- Horizontally-Curved Members
 - Introduction
 - Structural analysis
 - Flexural strength
 - Torsional strength (continued)




Photograph courtesy of the AISC Bender/Roller Committee



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Session Description

- Combined loads
- Serviceability
- Local strength
- Connections




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Design of Horizontally-Curved Members

Introduction



Introduction

- Curved Members \neq Straight Members
- Curved beam \rightarrow Flexure + Torsion

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Introduction

Deflected Shape

- Vertical translation
- Torsional rotation

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Introduction

Typical Behavior

- Dependent on θ_s
- $\theta_s < 1^\circ$: Flexure (F)
- $1^\circ \leq \theta_s \leq 20^\circ$: F + T
- $20^\circ < \theta_s$: Torsion (T)

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Introduction

Typical Limit States

- Excessive deformation
- Yielding

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Introduction

Efficient Structural Systems

- Infill members restrain torsion

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Introduction

Efficient Structural Systems

- Continuity
 - Flexure
 - Warping

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Design of Horizontally-Curved Members

Structural Analysis

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Structural Analysis

Analysis Methods


- Eccentric load method
- M/R method
- Finite element models

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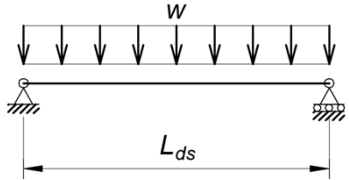

Design of Horizontally-Curved Members

Structural Analysis
 Eccentric Load Method



Eccentric Load Method

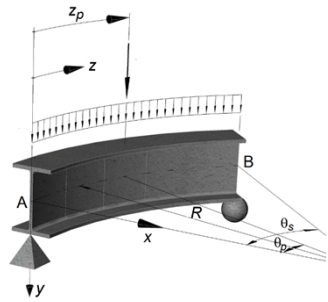
- Curved beam is modeled as straight member


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Eccentric Load Method

- Length is equal to the developed span length,
 $L_{ds} = R\theta_s$

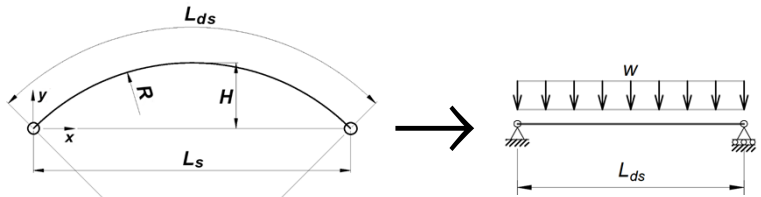



$\theta_s = \text{span angle, rad}$



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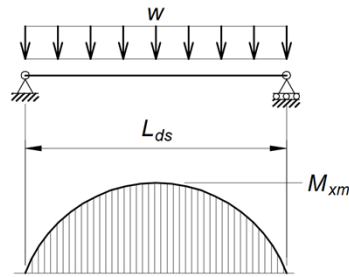
Eccentric Load Method

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Eccentric Load Method

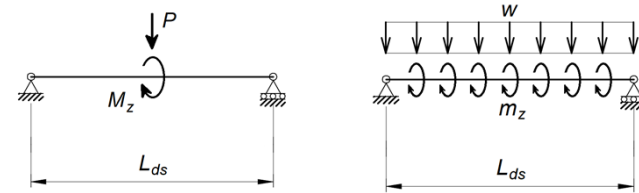
- Flexural (M_x) and shear (V) loads are calculated using the flexural support conditions



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Eccentric Load Method

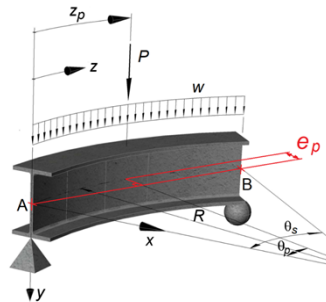
- Torsional moment = (vertical load) \times (horizontal eccentricity)



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Eccentric Load Method

- Eccentricity = distance between the load centroid and the chord
- Chord = straight line drawn between supports

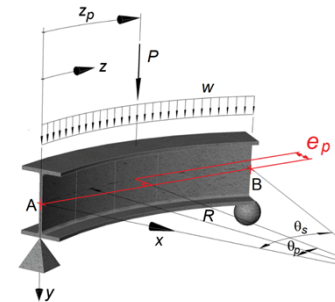


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Eccentric Load Method

- For beams with a midspan concentrated load, the midspan concentrated torsion is

$$M_z = Pe_p$$

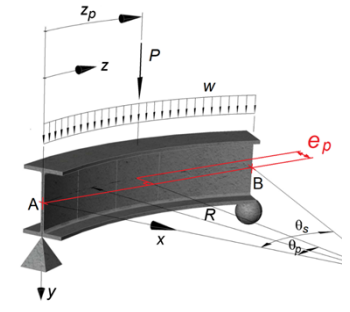


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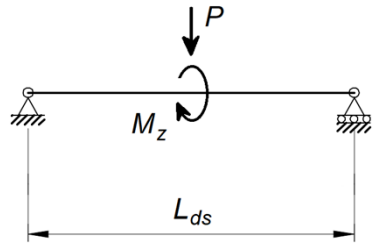
Eccentric Load Method

$$e_p = R \left[1 - \cos \left(\frac{\theta_s}{2} \right) \right]$$

θ_s = span angle, rad



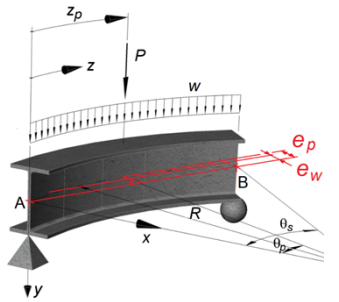
Eccentric Load Method



Eccentric Load Method

- For beams with a uniformly distributed load, the uniformly distributed torsion is

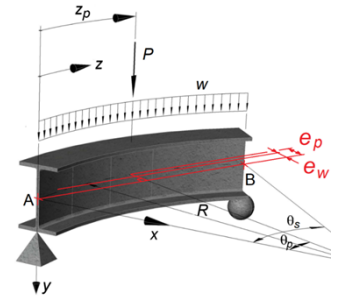
$$m_z = w e_w$$



Eccentric Load Method

$$e_w = R \left[\cos \left(\frac{\theta_s}{4} \right) - \cos \left(\frac{\theta_s}{2} \right) \right]$$


θ_s = span angle, rad



Eccentric Load Method

The diagram shows a horizontal beam of length L_{ds} supported at both ends. A uniformly distributed load W is applied downwards. A torsional moment m_z is applied along the length of the beam, represented by curved arrows.

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Design of Horizontally-Curved Members

Structural Analysis

M/R Method




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M/R Method

- Curved beam is modeled as straight member

The diagram shows a straight horizontal beam of length L_{ds} supported at both ends. A uniformly distributed load W is applied downwards.

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M/R Method

- Developed lengths
 - Span length: L_{ds}
 - Length between torsional restraints: L_{db}

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M/R Method

- Developed span length,
 $L_{ds} = R\theta_s$


$\theta_s = \text{span angle, rad}$



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M/R Method

- Flexural (M_x) and shear (V) loads are calculated using L_{ds} with the flexural support conditions




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M/R Method


- Developed length between torsional restraints, $L_{db} = R\theta_b$

$\theta_b = \text{angle between braces, rad}$

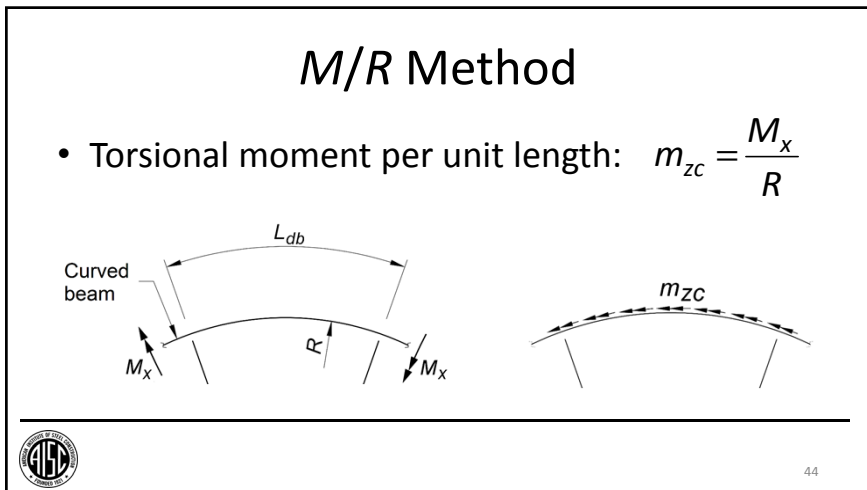
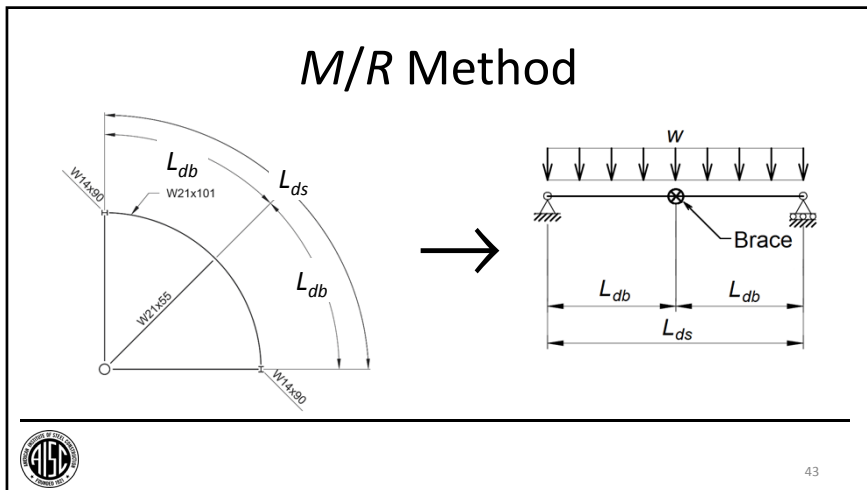
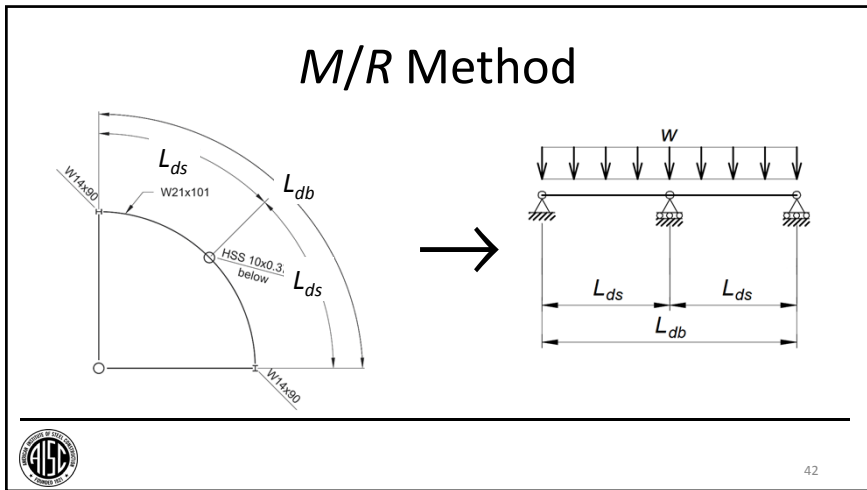
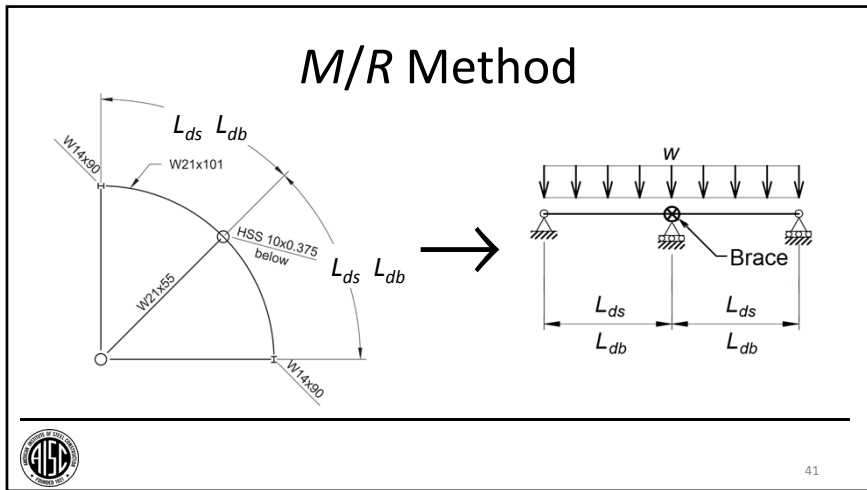


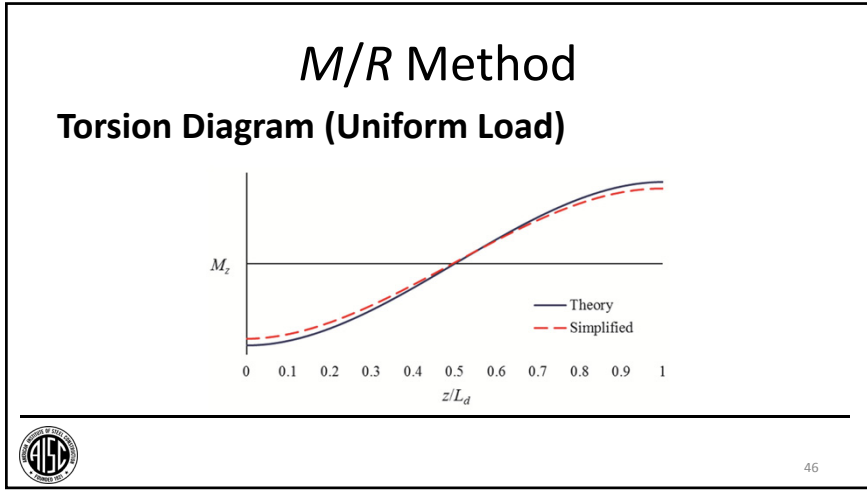
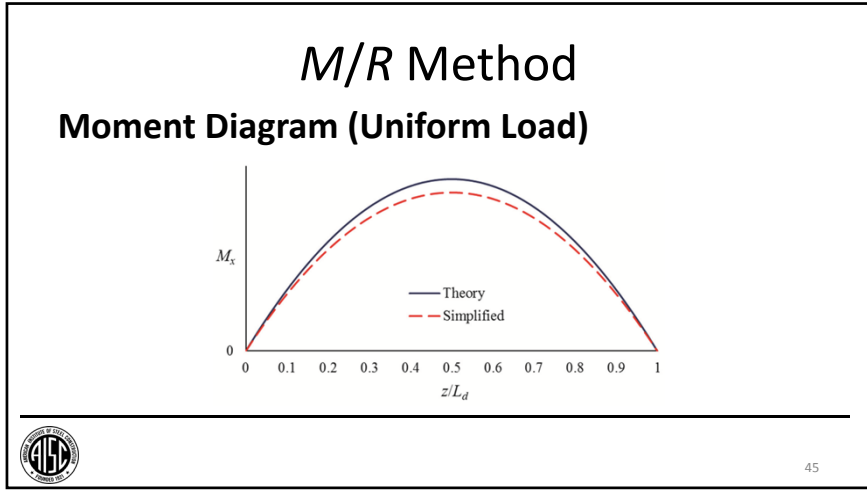
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M/R Method



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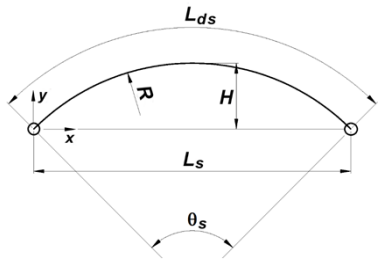
M/R Method


Corrected Moments

- Flexure: $M_{xc} = CM_x$
- Torsion: $M_{zc} = CM_z$

$$C = 1 - \frac{\theta_s}{30} + \frac{\theta_s^2}{6.2}$$

$\theta_s = \text{span angle, rad}$





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Design of Horizontally-Curved Members

Structural Analysis

Finite Element Models

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Finite Element Models

- Curved members are usually modeled by segmenting a series of straight elements
- Accuracy increases with the number of elements



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Finite Element Models

- \approx 10 to 20 segments for semi-circular members
- For models with highly nonlinear behavior, a convergence study may be required



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Finite Element Models

- 2-D Models: cross section is a single beam element
- 3-D Models: cross section is comprised of multiple elements (beam, plate, solid)



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Finite Element Models

2-D Models

- Basic beam finite element formulation
 - Used in most commercial finite element programs
 - Only St Venant stiffness
 - No warping stiffness
 - Over-estimates the rotation of open sections



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Finite Element Models

2-D Models

- The additional stiffness from warping can be approximated with an equivalent torsion constant, J_e

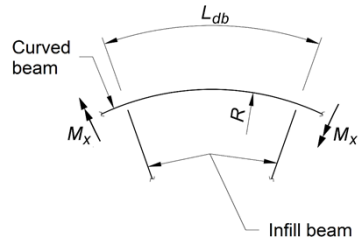


Ahmed, M.Z and Weisgerber, F.E. (1996), "Torsion Constant for Matrix Analysis of Structures Including Warping Effect," *International Journal of Solids and Structures*, Vol. 33, No. 3.

Finite Element Models

2-D Models

- Warping fixed at both ends of the span



Ahmed, M.Z and Weisgerber, F.E. (1996), "Torsion Constant for Matrix Analysis of Structures Including Warping Effect," *International Journal of Solids and Structures*, Vol. 33, No. 3.

Finite Element Models

2-D Models

$$J_e = \frac{J}{1 - \frac{\sinh \gamma}{\gamma} + \frac{(\cosh \gamma - 1)^2}{\gamma \sinh \gamma}}$$



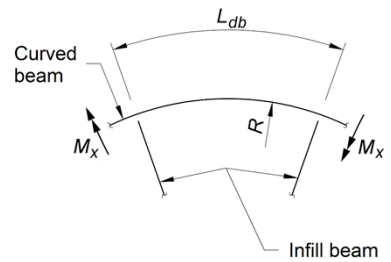
Ahmed, M.Z and Weisgerber, F.E. (1996), "Torsion Constant for Matrix Analysis of Structures Including Warping Effect," *International Journal of Solids and Structures*, Vol. 33, No. 3.

Finite Element Models

2-D Models

$$\gamma = L_{db} \sqrt{\frac{GJ}{EC_w}}$$

C_w = warping constant
 J = torsional constant



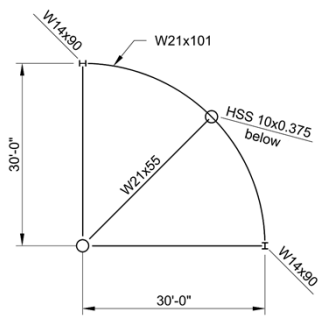
Ahmed, M.Z and Weisgerber, F.E. (1996), "Torsion Constant for Matrix Analysis of Structures Including Warping Effect," *International Journal of Solids and Structures*, Vol. 33, No. 3.



Finite Element Models

2-D Models

- Warping fixed at one end and free at one end



Ahmed, M.Z and Weisgerber, F.E. (1996), "Torsion Constant for Matrix Analysis of Structures Including Warping Effect," *International Journal of Solids and Structures*, Vol. 33, No. 3, January, pp 361-374. 57

Finite Element Models

2-D Models

$$J_e = \frac{J}{1 - \frac{\sinh \gamma}{\gamma \cosh \gamma}}$$

Ahmed, M.Z and Weisgerber, F.E. (1996), "Torsion Constant for Matrix Analysis of Structures Including Warping Effect," *International Journal of Solids and Structures*, Vol. 33, No. 3, January, pp 361-374. 58

Finite Element Models

3-D Models

- Cross section is comprised of multiple elements

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Finite Element Models

3-D Models


- Webs of I-shape members
 - Typically modeled with plate elements
 - Can be modeled with solid elements

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Finite Element Models

3-D Models

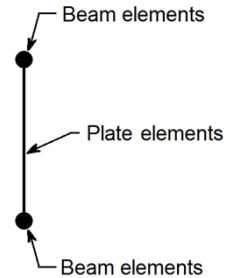
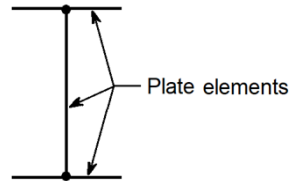
- Flanges of I-shape members
 - Typically modeled with beam elements
 - Can be modeled with plate or solid elements



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Finite Element Models

3-D Models




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Finite Element Models

3-D Models

- Infill members
 - Typically modeled with beam elements
 - Can be modeled with plate or solid elements

(continued)




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Finite Element Models

3-D Models

- If used to restrain torsion, connect to nodes at the top and bottom flanges of the curved member



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Finite Element Models

3-D Models

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Design of Horizontally-Curved Members

Flexural Strength

There's always a solution in steel.

Flexural Strength

- Design as a straight beam
- AISC *Specification* Chapter F
- Unbraced length, $L_b \rightarrow L_{db}$
- Lateral-torsional buckling modification factor, $C_b \rightarrow C_{bo}$

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Flexural Strength

$$C_{bo} = C_{bs} \left[1 - \left(\frac{\theta_b}{\pi} \right)^2 \right]^2$$

θ_b = angle between braces, rad
 $C_{bs} = C_b$ for an equivalent straight member


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Adapted from: Yoo, C.H., Kang, Y.J. and Davidson, J.S. (1996), "Buckling Analysis of Curved Beams by Finite-Element Discretization," *Journal of Engineering Mechanics*, ASCE, Vol. 122, No. 8.

There's always a solution in steel.


Design of Horizontally-Curved Members

Torsional Strength



Torsional Strength

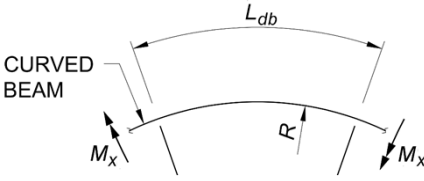
- Design as a straight beam
- Properly account for end conditions
 - Warping fixed
 - Warping free




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Torsional Strength

- Member length = developed length between torsional restraints, L_{db}




CURVED BEAM



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Torsional Strength

- Analysis Methods
 - Elastic method
 - Isolated flange method




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Design of Horizontally-Curved Members


Torsional Strength

Elastic Method





Elastic Method

- AISC Design Guide 9
- Design charts in Appendix B



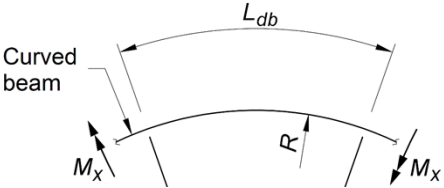

Torsional Analysis of Structural Steel Members

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Elastic Method

Equal End Moments

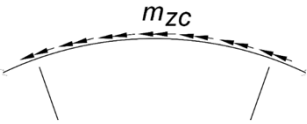




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Elastic Method

Equal End Moments

- Results in a uniformly distributed torsional moment

$$m_{zc} = \frac{M_x}{R}$$



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Elastic Method

Equal End Moments

- Design Guide 9 Charts
 - Warping free → Case 4
 - Warping fixed → Case 7
 - Warping fixed/free → Case 12

Case 4 $\psi = \left(\frac{I_w}{I_y I_z}\right)^{0.5}$

Normalized Moment vs. Fraction of Span Length x/L

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Design of Horizontally-Curved Members

Torsional Strength

Isolated Flange Method

There's always a solution in steel.

Isolated Flange Method

- I-shaped members
- Flanges are modeled as independent rectangular beams

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Isolated Flange Method

Top flange

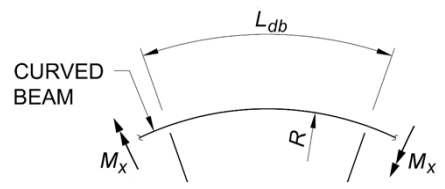
Radial Load

Moment Diagram

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Isolated Flange Method

- Length of the isolated flange = developed (arc) length between torsional restraints, L_{db}



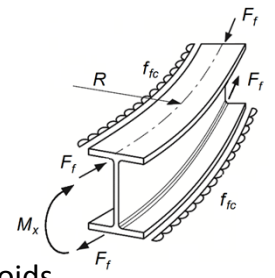
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Isolated Flange Method

- Radial load, f_{fc} , is applied in the horizontal plane

$$f_{fc} = \frac{m_{zc}}{h_o} = \frac{M_x}{Rh_o}$$

h_o = distance between flange centroids



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Isolated Flange Method

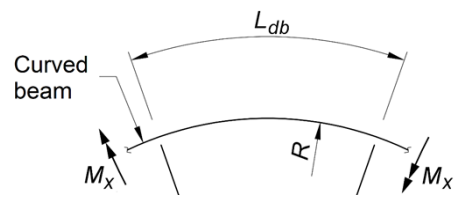
- Flexural boundary conditions of the isolated flange are based on the warping boundary conditions of the curved member



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Isolated Flange Method

Equal End Moments

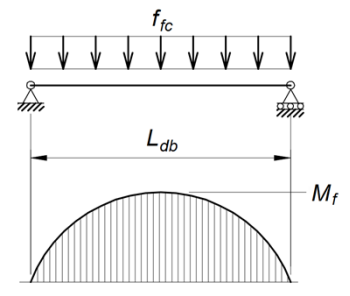


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Isolated Flange Method

Equal End Moments

- Moment diagram for the isolated flange
- Warping free

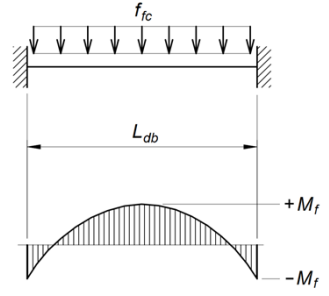


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Isolated Flange Method

Equal End Moments

- Moment diagram for the isolated flange
- Warping fixed



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Isolated Flange Method

- Nominal flexural strength of the isolated flange:

$$M_{nw} = F_y Z_f$$

$$Z_f = \frac{t_f b_f^2}{4}$$

b_f = flange width
 t_f = flange thickness



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Design of Horizontally-Curved Members

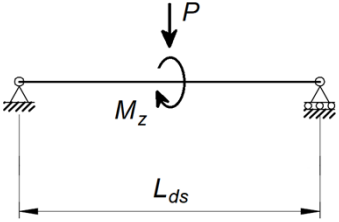
Combined Loads

There's always a solution in steel.




Combined Loads

Flexure-torsion Interaction



The diagram shows a horizontal beam of length L_{ds} supported at both ends by pin supports. A downward point load P is applied at the center of the beam. A clockwise moment M_z is also applied at the center. The beam is shown in its undeformed state.




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Design of Horizontally-Curved Members

Combined Loads

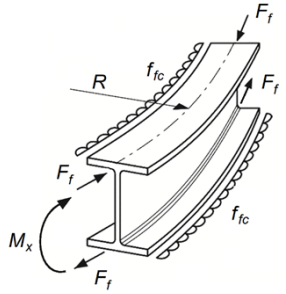
Second-Order Effects




There's always a solution in steel.

Second-Order Effects

- Increase torsional moments and torsional rotations
- Isolated compression flange is analogous to a beam-column




The diagram shows a curved beam with a radius R . It is subjected to a moment M_x and forces F_f at its ends. The diagram illustrates the internal stresses f_{fc} in the compression flange, which is shown as a curved beam-column.



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Second-Order Effects

- Rigorous second-order analysis
- Amplified first-order analysis




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Second-Order Effects

Amplified First-Order Analysis

- Open sections subjected to torsion + strong-axis flexure



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Second-Order Effects

Amplified First-Order Analysis

- Second-order torsional rotation: $\theta_2 = B_o \theta_1$
- Second-order torsional moment: $M_{rz} = B_o M_z$

M_z = first-order torsional moment
 θ_1 = first-order torsional rotation



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Second-Order Effects

Amplified First-Order Analysis

$$B_o = \frac{0.85}{1 - \alpha \frac{M_{ro}}{M_{eo}}} \geq 1.0$$

M_{eo} = elastic lateral-torsional buckling moment
 M_{ro} = required strong-axis flexural moment (M_{rx})
 α = 1.00 (LRFD); 1.60 (ASD)



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Design of Horizontally-Curved Members

Combined Loads

Member Strength

There's always a solution in steel.



Member Strength

HSS and Box-Shaped Members

- Round and rectangular HSS
- Built-up box-shaped members
- *Specification* Section H3.2, Equation H3-6



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Member Strength

HSS and Box-Shaped Members

$$\frac{M_{ro}}{M_{co}} + \left(\frac{V_r}{V_c} + \frac{M_{rz}}{M_{cz}} \right)^2 \leq 1.0$$

M_{co} M_{cz} V_c = available strengths: flexure, torsion, shear

M_{ro} M_{rz} V_r = required strengths: flexure, torsion, shear



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Member Strength

I-Shaped Members

- Interaction method is based on the analysis method
 - Isolated flange method
 - Elastic method
 - FE model



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Member Strength

I-Shaped Members: Isolated Flange Method

$$\frac{M_{ro}}{M_{co}} + \frac{8 M_{rw}}{9 M_{cw}} \leq 1.0$$

M_{co} M_{cw} = available flexural strengths: member, flange

M_{ro} M_{rw} = required flexural strength: member, flange




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Member Strength

I-Shaped Members: Elastic Method

$$\frac{\sigma_{ro}}{\sigma_{co}} + \frac{16 \sigma_{rw}}{27 \sigma_{cw}} \leq 1.0$$

σ_{co} σ_{cw} = available stresses: flexure, warping
 σ_{ro} σ_{rw} = required stresses: flexure, warping




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Member Strength

I-Shaped Members: 2-D FE Model

- The required loads from the model can be compared with the available loads in the AISC *Specification* and AISC Design Guide 9




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Member Strength

I-Shaped Members: 3-D FE Model

- Flange strength is evaluated using AISC *Specification* Section H1

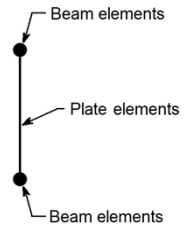



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Member Strength

I-Shaped Members: 3-D FE Model

- Flanges modeled as rectangular beam elements: loads are used directly from the FE analysis





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Member Strength

I-Shaped Members: 3-D FE Model

- The available strengths in the AISC *Specification* were not developed to be compared to the results from finite element models built with plate or solid elements

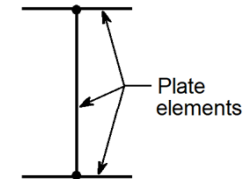


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Member Strength

I-Shaped Members: 3-D FE Model

- Flanges modeled as plate or solid elements: loads are determined by summing stresses over the element



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Design of Horizontally-Curved Members

Serviceability

There's always a solution in steel.



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Serviceability

- Large deformations at ultimate strength
- Member sizes are usually controlled by serviceability

Serviceability

- Torsional rotation limits
 - Not in building codes
 - Some judgment may be required to define appropriate limits
 - Maintain geometry of the structure
 - Prevent damage to nonstructural elements



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Design of Horizontally-Curved Members

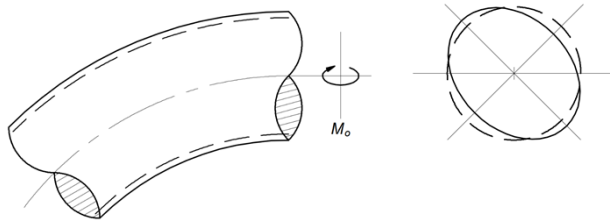
Local Strength

There's always a solution in steel.



Local Strength

Out-of-Plane Moment



111

Simplified Method

Effective Flexural Properties (Round HSS)

- Effective section modulus: $S_{eo} = k_{so} S$
- Effective plastic modulus: $Z_{eo} = k_{zo} Z$
- Effective moment of inertia: $I_e = k_i I$

I, S, Z = flexural properties of a straight member




112

Simplified Method

Effective Flexural Properties (Round HSS)

$$k_{so} = 0.926c_r^{2/3} \leq 1.00 \qquad c_r = \frac{4Rt}{(D-t)^2}$$


$D =$ outside diameter $t =$ wall thickness


113

Simplified Method

Effective Flexural Properties (Round HSS)


$$k_{zo} = \frac{1.2c_r}{\sqrt{1+c_r^2}} \qquad k_i = \frac{1}{1 + \frac{9}{12c_r^2 + 1}}$$


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There's always a solution in steel.

Design of Horizontally-Curved Members

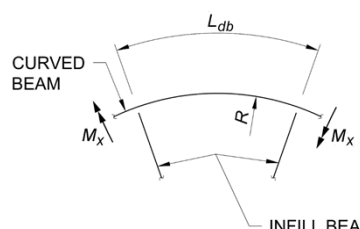

Connections




AMERICAN INSTITUTE OF STEEL CONSTRUCTION
 structural STEEL

Connections

- Torsional resistance is required for equilibrium


116

Connections

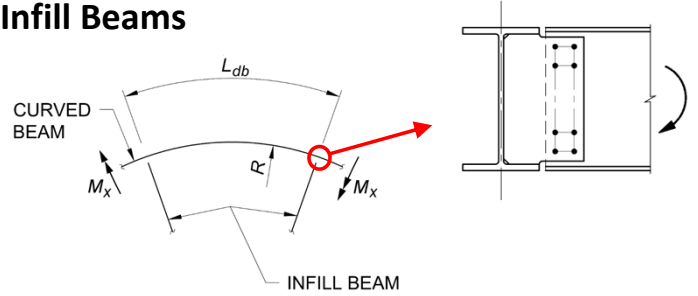
- Warping resistance is optional
 - Increased member twisting strength and stiffness
 - Dependent on connections
 - Often impractical due to connection requirements



117

Connections

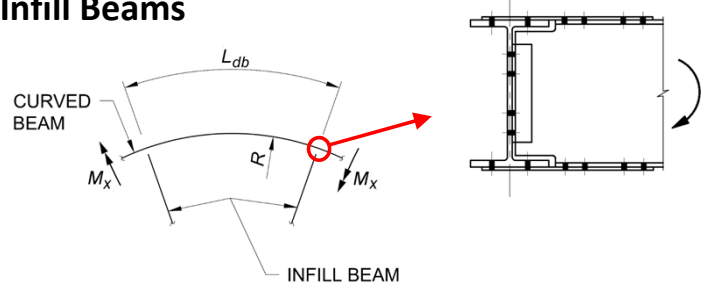
Infill Beams



118

Connections

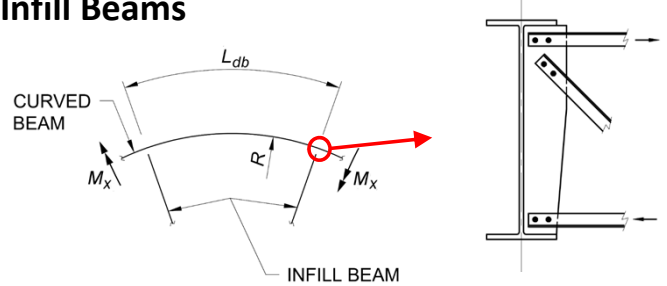
Infill Beams



119

Connections

Infill Beams



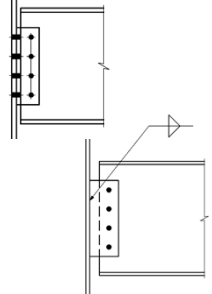
120




Connections

End Connections

- Simple shear connections
→ limited torsional resistance



The diagram shows two steel members connected at their ends. The top member is a wide flange beam, and the bottom member is a narrower flange beam. They are connected by a vertical web plate with several bolts. The connection is a simple shear connection, where the bolts are in shear. A dashed line indicates the axis of the top member, and a solid line indicates the axis of the bottom member. The connection is shown in a perspective view.

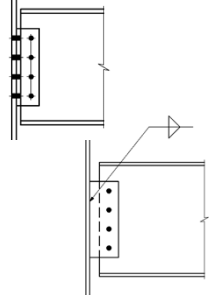


121


Connections

End Connections

- Flanges must be engaged to properly transfer torsional loads



The diagram shows two steel members connected at their ends. The top member is a wide flange beam, and the bottom member is a narrower flange beam. They are connected by a vertical web plate with several bolts. The connection is a flange-to-flange connection, where the flanges of the two members are engaged. A dashed line indicates the axis of the top member, and a solid line indicates the axis of the bottom member. The connection is shown in a perspective view.

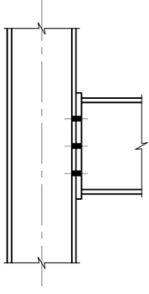


122


Connections

End Connections

- Warping free



The diagram shows two steel members connected at their ends. The top member is a wide flange beam, and the bottom member is a narrower flange beam. They are connected by a vertical web plate with several bolts. The connection is a warping free connection, where the flanges of the two members are not engaged. A dashed line indicates the axis of the top member, and a solid line indicates the axis of the bottom member. The connection is shown in a perspective view.

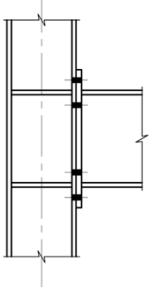


123


Connections

End Connections

- Warping fixed




The diagram shows two steel members connected at their ends. The top member is a wide flange beam, and the bottom member is a narrower flange beam. They are connected by a vertical web plate with several bolts. The connection is a warping fixed connection, where the flanges of the two members are engaged. A dashed line indicates the axis of the top member, and a solid line indicates the axis of the bottom member. The connection is shown in a perspective view.



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There's always a solution in steel.

Question time



Individual Webinar Registrants

CEU/PDH Certificates

Within 2 business days...

- You will receive an email on how to report attendance from: registration@aisc.org.
- Be on the lookout: Check your spam filter! Check your junk folder!
- Completely fill out online form. Don't forget to check the boxes next to each attendee's name!




Individual Webinar Registrants

CEU/PDH Certificates

Within 2 business days...

- New reporting site (URL will be provided in the forthcoming email).
- Username: Same as AISC website username.
- Password: Same as AISC website password.



8-Session Registrants

CEU/PDH Certificates

One certificate will be issued at the conclusion of all 8 sessions.



8-Session Registrants

Access to the quiz: Information for accessing the quiz will be emailed to you by Wednesday. It will contain a link to access the quiz. EMAIL COMES FROM NIGHTSCHOOL@AISC.ORG

Quiz and Attendance records: Posted Tuesday mornings.
www.aisc.org/nightsschool - click on Current Course Details.

Reasons for quiz:

- EEU – must take all quizzes and final to receive EEU
- CEUs/PDHS – If you watch a recorded session you must take quiz for CEUs/PDHS.
- REINFORCEMENT – Reinforce what you learned tonight. Get more out of the course.

NOTE: If you attend the live presentation, you do not have to take the quizzes to receive CEUs/PDHS.



8-Session Registrants

Access to the recording: Information for accessing the recording will be emailed to you by this Wednesday. The recording will be available for three weeks. For 8-session registrants only. EMAIL COMES FROM NIGHTSCHOOL@AISC.ORG.

CEUs/PDHS – If you watch a recorded session you must take AND PASS the quiz for CEUs/PDHS.



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Night School Resources for 8-session package Registrants

Night School 13: Design of Industrial Buildings

8-SESSION PACKAGE RESOURCES

Event	Date	Handouts	Video	Quiz	Attendance
NS13 - Design Criteria	1/30/2017 7:00:00 PM	Handouts	Video	Pass Score: 80	Pending
NS13 - Economic Considerations	2/6/2017 7:00:00 PM	Handouts	Available 02/06/2017 5pm EST	Available 02/08/2017 5pm EST	Pending
NS13 - Lateral Load Systems and Details	2/13/2017 7:00:00 PM	Handouts	Available 02/13/2017 5pm EST	Available 02/15/2017 5pm EST	Pending
NS13 - Preliminary Design Procedures	2/27/2017 7:00:00 PM	Handouts	Available 03/01/2017 5pm EST	Available 03/01/2017 5pm EST	Pending
NS13 - Crane Girder Design and Frame Analysis	3/6/2017 7:00:00 PM	Handouts	Available 03/06/2017 5pm EST	Available 03/08/2017 5pm EST	Pending
NS13 - Frame Member and Connection Design	3/13/2017 7:00:00 PM	Handouts	Available 03/13/2017 5pm EST	Available 03/15/2017 5pm EST	Pending
NS13 - Transfer Crane Girder & Longitudinal Brag Bracing Dgn	3/27/2017 7:00:00 PM	Handouts	Available 03/28/2017 5pm EST	Available 03/29/2017 5pm EST	Pending
NS13 - Building Envelope and Bracing Design	4/3/2017 7:00:00 PM	Handouts	Available 04/05/2017 5pm EST	Available 04/05/2017 5pm EST	Pending
NS13 - Final Exam	4/10/2017 7:00:00 PM			Available 04/10/2017 5pm EST	

Night School Resources for 8-session package Registrants

- Weekly “quiz and recording” email.
- Weekly updates of the master Quiz and Attendance record found at www.aisc.org/nightschool. Scroll down to Quiz and Attendance records.
 - Updated on Tuesday mornings.

Night School Resources for 8-session package Registrants

- Webinar connection information:
 - Found in your registration confirmation/receipt.
 - Reminder email sent out Monday mornings.
- Link to handouts also found here.

There's always a solution in steel.

Thank You

Please give us your feedback!
Survey at conclusion of webinar.

